

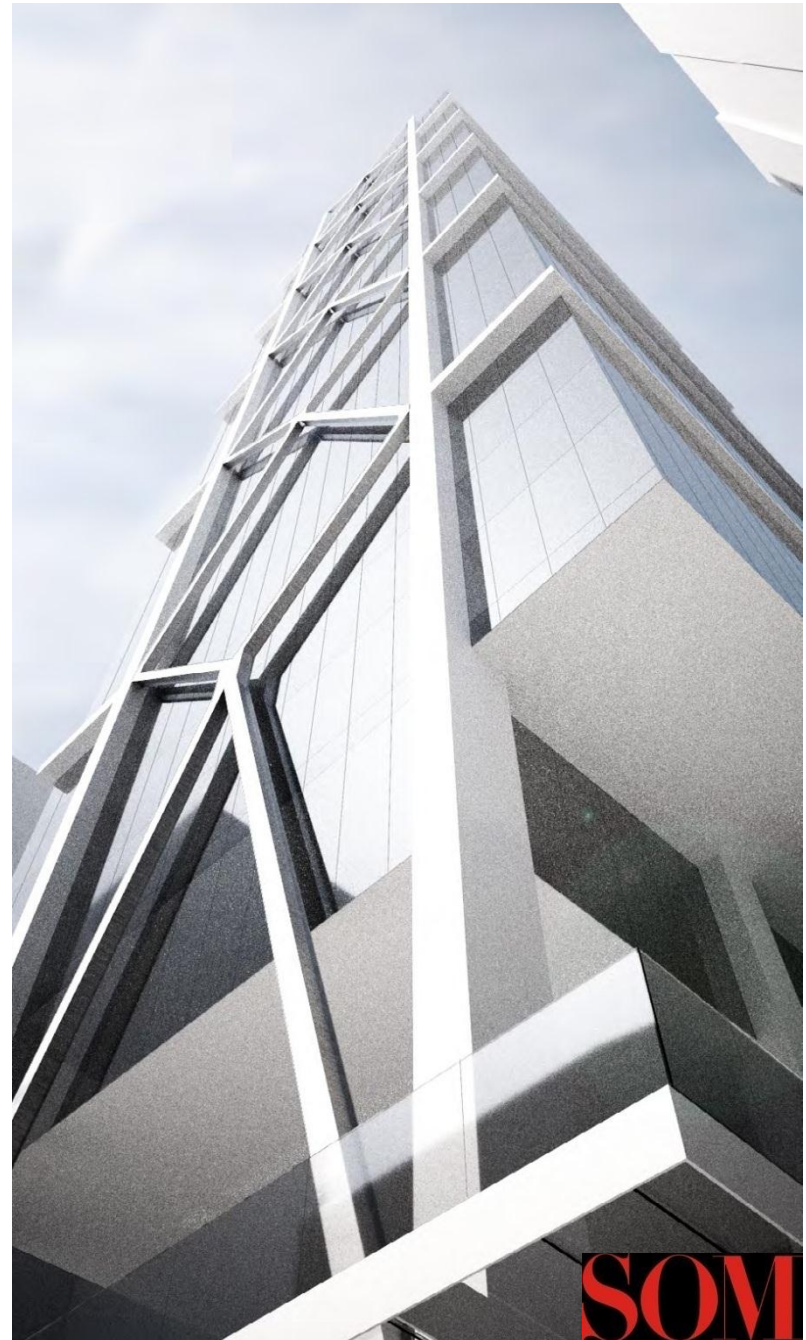
Design of Structural Braced Frames Using Topology Optimization

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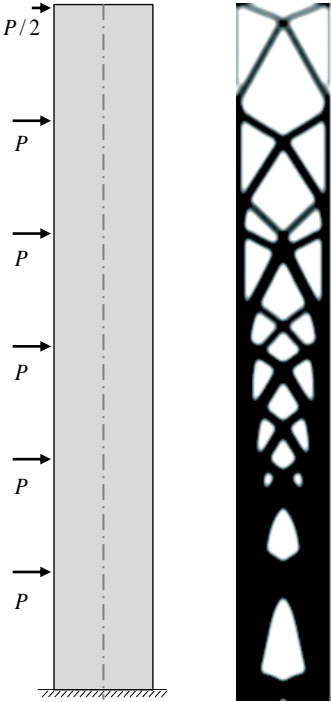
March 30, 2012



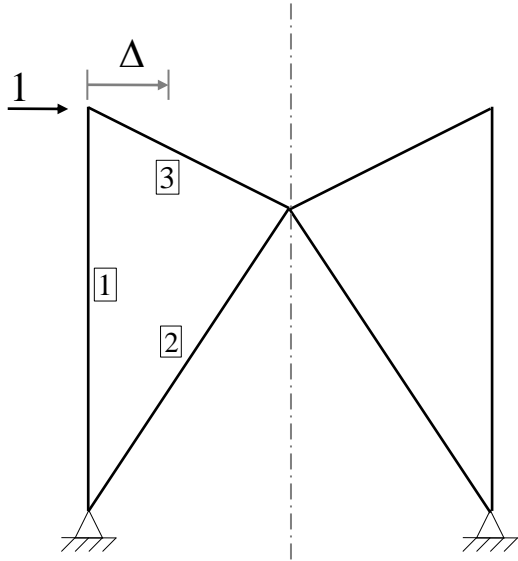
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This presentation focuses on the use of topology optimization with combined elements in the building design industry



Motivation for new approach



Analytical aspects



Combining elements for structural braced frames



Architecture without evident engineering rationale is wasteful of resources and difficult to realize

Railway



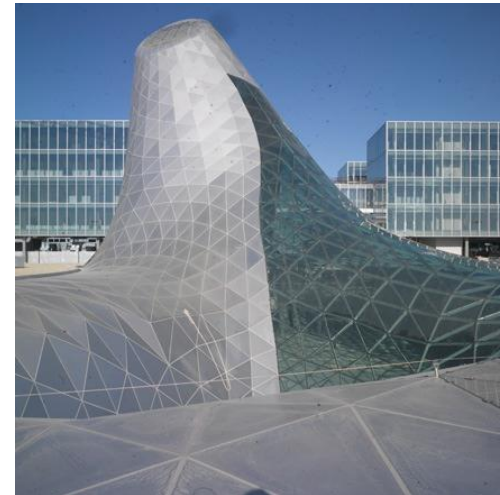
<http://en.wikipedia.org/wiki/Hadid-Afragola>

Olympic Stadium



http://en.wikipedia.org/wiki/Beijing_National_Stadium

Convention Center



<http://images.businessweek.com>



Buildings can be designed using the lateral bracing for both engineering and architectural criteria



http://en.wikipedia.org/wiki/John_Hancock_Center



http://en.wikipedia.org/wiki/Broadgate_Tower



http://en.wikipedia.org/wiki/Bank_of_China



Stromberg, L.L., Beghini, A., Baker, W.F., Paulino, G.H. "Topology optimization for braced frames: Combining continuum and beam/column elements." *Engineering Structures* 2012, 37:106-124.



Topology optimization gives practical designs that satisfy principles from both structural engineering and architecture



*Images courtesy of SOM

Design for the “bridges” spanning between towers for the Zendai competition was a collaborative effort between UIUC and SOM



Pallasmaa, J., ed. *SOM Journal 7*. Ostfildern: Hatje Cantz Verlag, 2012.



Resulting designs resemble patterns from nature



Photography.nationalgeographic.com

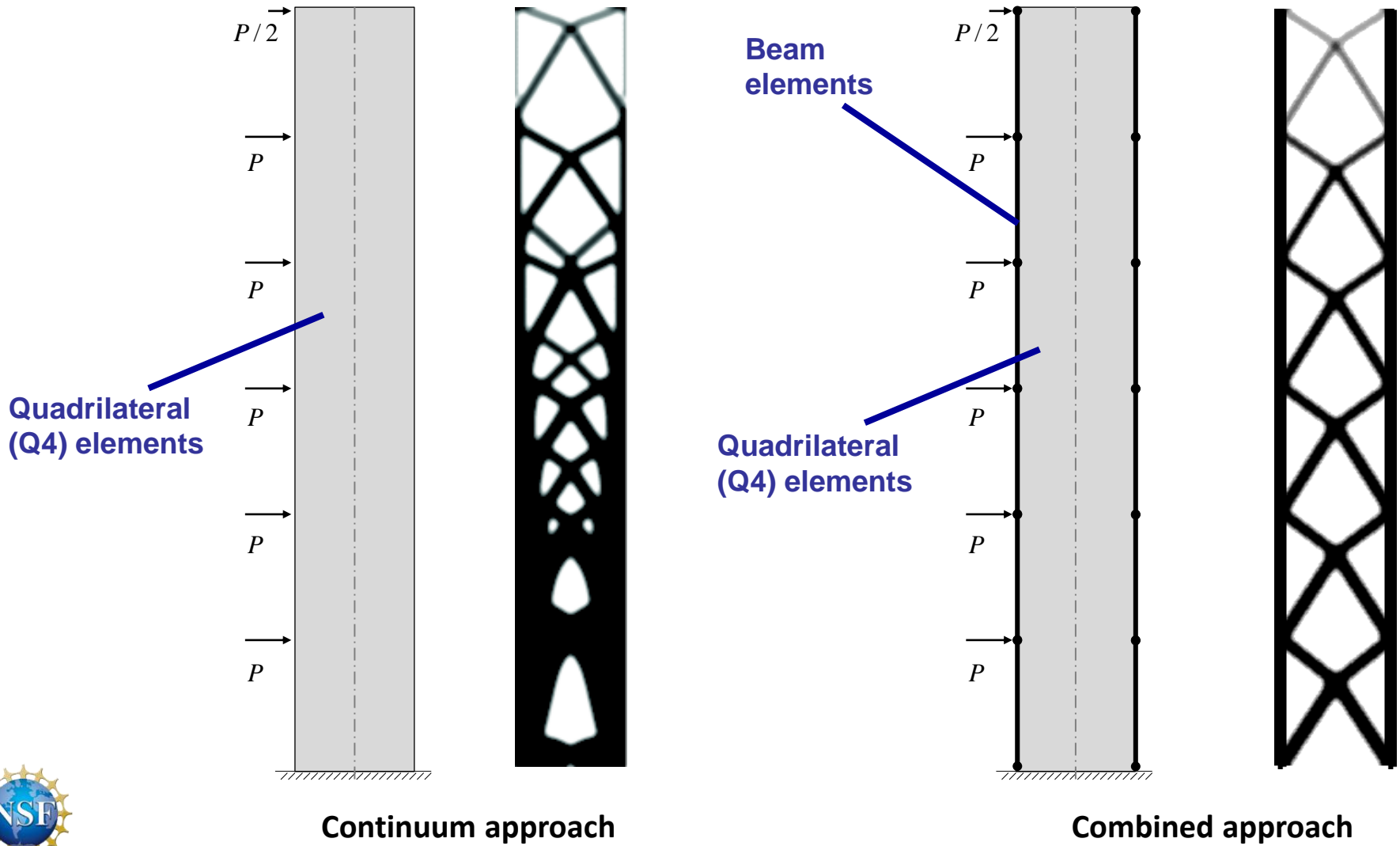
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Current topology optimization techniques demonstrate limitations in design



Proposed Design Process

1. Size vertical line elements according to gravity load combinations using technique of Baker (1992)¹
2. Run topology optimization on the continuum elements for lateral load combinations (accounting for wind and seismic loads)
3. Identify optimal bracing layout to create frame model
4. Optimize final member sizes using virtual work methodology¹
5. Iterate (if necessary)

¹Baker, W.F. "Energy-based design of lateral systems" *Struct Eng Int* 1992, 2:99-102.



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Optimal design results in a state of constant stress

In terms of displacements u_i at each point of load application P_i the compliance can be expressed as

$$W_{ext} = \sum_i P_i u_i = \sum_j \frac{N_j^2 L_j}{EA_j} = W_{int}$$

By introducing the Lagrangian multiplier constraint on the volume,

$$W_{ext} = \sum_j \frac{N_j^2 L_j}{EA_j} + \lambda \left(\sum_j A_j L_j - V \right)$$

Differentiating with respect to the areas A_i and solving for the Lagrangian multiplier,

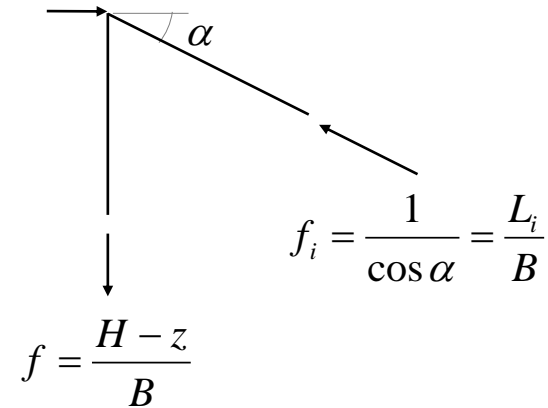
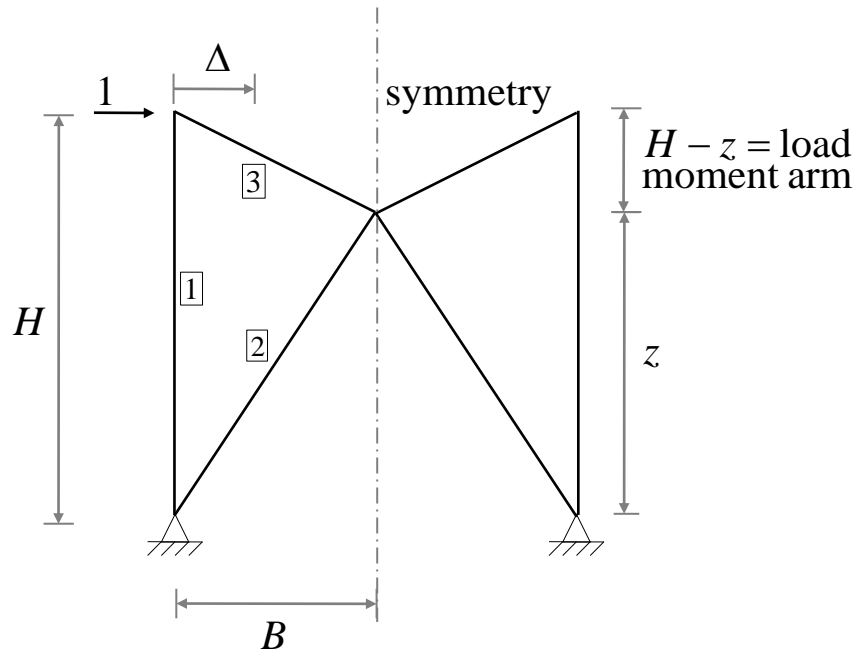
$$\lambda = \left(\frac{N_i}{A_i} \right)^2 \frac{1}{E} = \frac{\sigma^2}{E} = \text{const}$$



Optimal Single Module Bracing

The optimal bracing geometry for a single module is considered:

$$f_1 = \frac{H - z}{B}, f_2 = \frac{\sqrt{B^2 + z^2}}{B}, f_3 = \frac{-\sqrt{B^2 + (H - z)^2}}{B}$$



Optimal Single Module Bracing

Assuming each member to have a constant stress,

$$\Delta = \sum_i \frac{f_i F_i L_i}{E A_i} = \frac{\sigma B}{E} \sum_i \frac{f_i L_i}{B}$$

The tip deflection of the frame is minimal when

$$\begin{aligned} \frac{\partial \Delta}{\partial z} &= \frac{\sigma B}{E} \frac{\partial}{\partial z} \left(\sum_i \frac{f_i L_i}{B} \right) = 0 \\ &= \frac{\sigma B}{E} \frac{\partial}{\partial z} \left(H \left(\frac{H-z}{B^2} \right) + \frac{B^2 + z^2}{B^2} + \frac{B^2 + (H-z)^2}{B^2} \right) = 0 \end{aligned}$$

Thus, the brace work point height is optimal at

$$z = \frac{3}{4} H$$



These rules can be generalized for multiple modules

Generalizing the previous equations for N modules,

$$\begin{aligned}\Delta &= \frac{\sigma B}{E} \sum_i \frac{f_i L_i}{B} \\ &= \frac{\sigma B}{E} \left[\sum_i \left(\frac{L_i^2}{B^2} \right)_{braces} + \sum_j \left(\frac{(H - z_j) L_j}{B^2} \right)_{columns} \right] \\ &= \frac{\sigma B}{E} \left[\sum_{n=1}^N (z_{2n} - z_{2n-2}) \frac{H - z_{n-1}}{B^2} + \frac{B^2 + (z_{2n-1} - z_{2n-2})^2}{B^2} + \frac{B^2 + (z_{2n-1} - z_{2n-2})^2}{B^2} \right]\end{aligned}$$

Differentiating,

$$\begin{aligned}\frac{\partial}{\partial z_{2n-1}} \left(\frac{E\Delta}{\sigma B} \right) &= 0 \rightarrow -3z_{2n} + 4z_{2n-1} - z_{2n-2} = 0 \\ \frac{\partial}{\partial z_{2n}} \left(\frac{E\Delta}{\sigma B} \right) &= 0 \rightarrow -z_{2n+1} + 4z_{2n} - 3z_{2n-1} = 0\end{aligned}$$



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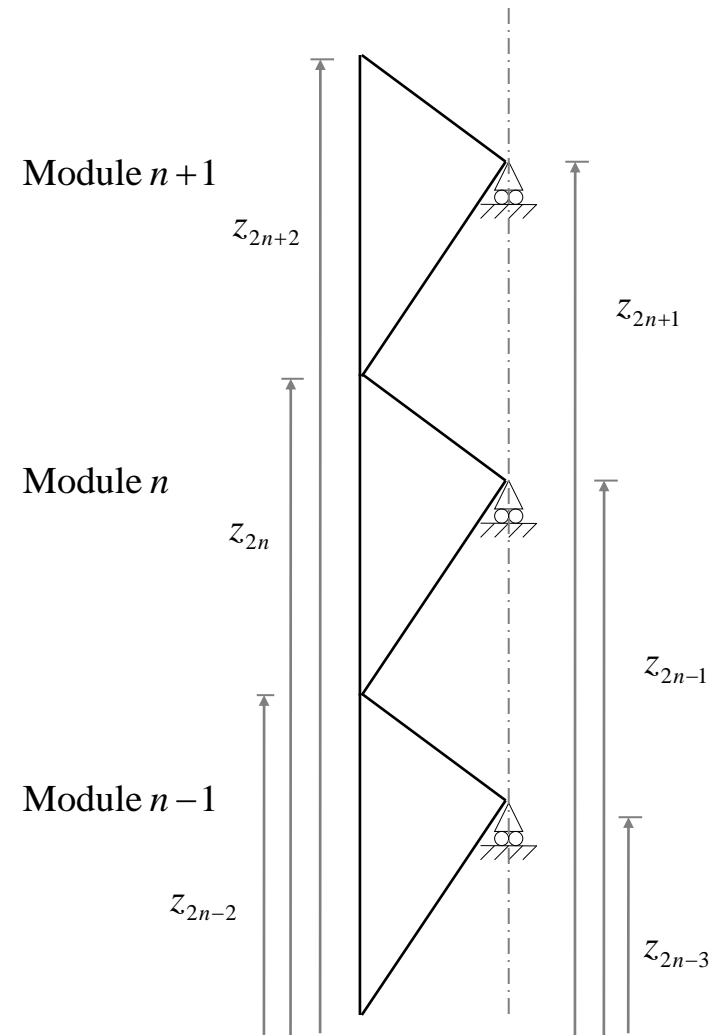
Optimal Multiple Modules Bracing

Rewriting these equations

$$z_{2n} = \frac{z_{2n-1} + z_{2n+1}}{2} - \frac{z_{2n+1} - z_{2n-1}}{4}$$
$$z_{2n-1} = \frac{z_{2n-2} + z_{2n}}{2} + \frac{z_{2n} - z_{2n-2}}{4}$$

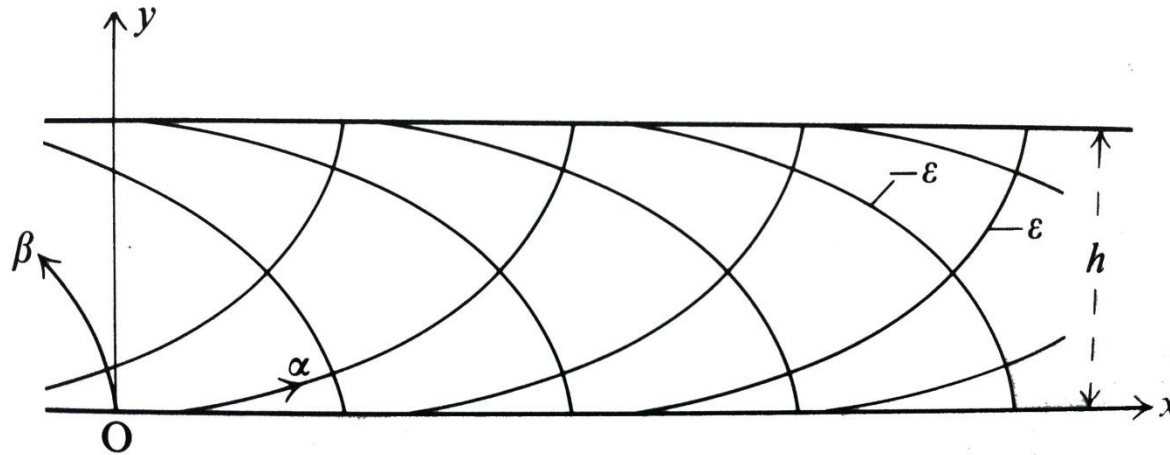
This leads to two conclusions:

1. The braced frame central work point is always at 75% of the module height.
2. The module heights are all equal, indicating that patterns are optimal.



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These results can be verified using Michell solutions



In 1904, Michell derived the mathematics behind structures of least volume, or *optimal structures*

Hemp discusses the example of a cantilever beam subject to a point load with solutions formed by cycloids using the conditions set forth in Michell (1904).



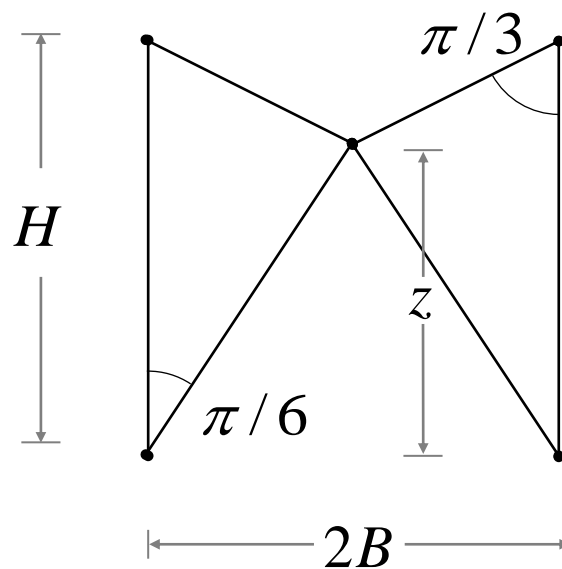
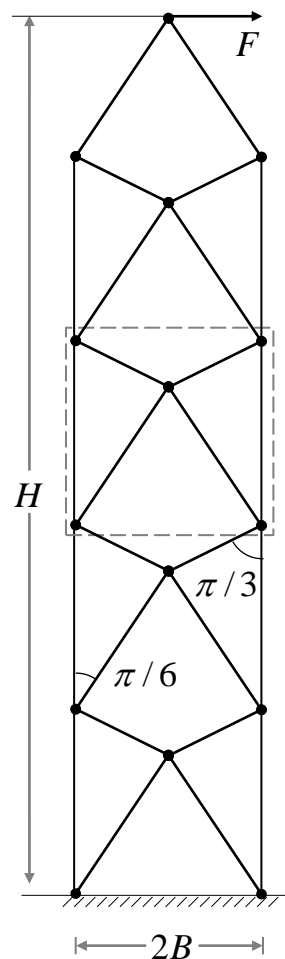
Michell, A.G.M. "The Limits of Economy of Material in Frame-Structures." *Phil Mag* 8(47):589-597

Hemp, W.M. *Optimum Structures*. Oxford: Clarendon Press, 1973.



These results can be verified using Michell solutions

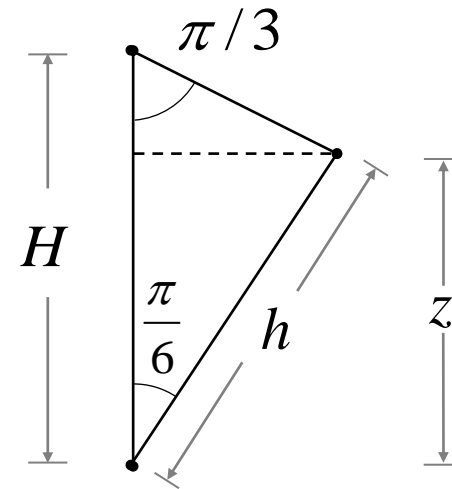
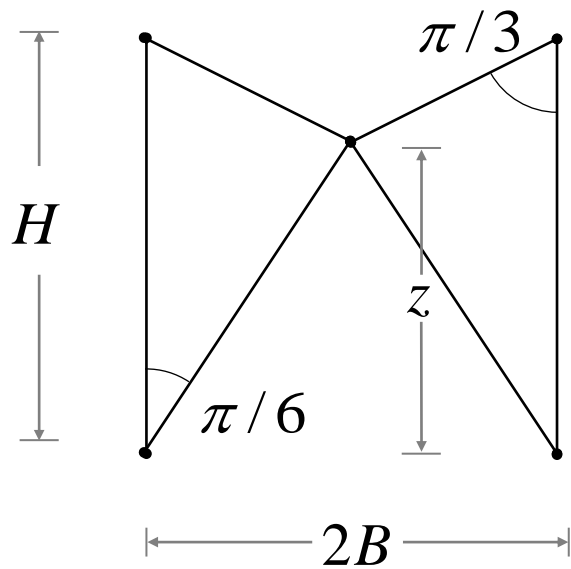
In Hemp (1973), an application of the Michell truss solution is given by the optimal shear bracing with the discretization of cycloids:



Optimal geometry given by angles – how does this correspond to the 75% rule?



These results can be verified using Michell solutions



$$h = H \cos(\pi / 6)$$

$$z = h \cos(\pi / 6) = H \left(\frac{\sqrt{3}}{2} \right)^2 = \boxed{\frac{3}{4} H}$$

Same results!



Topology Optimization Framework

Problem statement:

$\min_{\rho, \mathbf{u}} \quad c(\rho, \mathbf{u})$	Objective function
$s. t. : \quad \mathbf{K}(\rho)\mathbf{u} = \mathbf{f}$	Equilibrium constraint
$\int_{\Omega} \rho \, dV \leq V_s$	Volume constraint

Using Solid Isotropic Material with Penalization (SIMP) model,

$$E(\mathbf{x}) = \rho(\mathbf{x})^p E^0, \quad p > 1$$



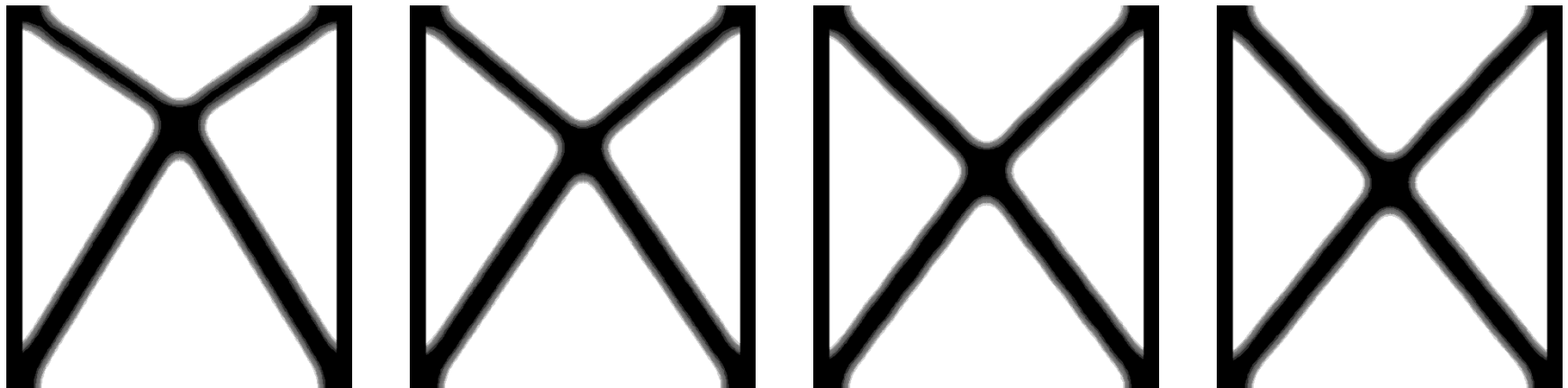
Bendsoe, M.P., Sigmund, O. *Topology Optimization: Theory, Methods and Applications*. Springer, 2002.



The optimal bracing point is highly influenced by the stress levels



Higher areas than constant stress state give lower bracing points



$$A = A_0$$

$$A = 2A_0$$

$$A = 5A_0$$

$$A = 10A_0$$

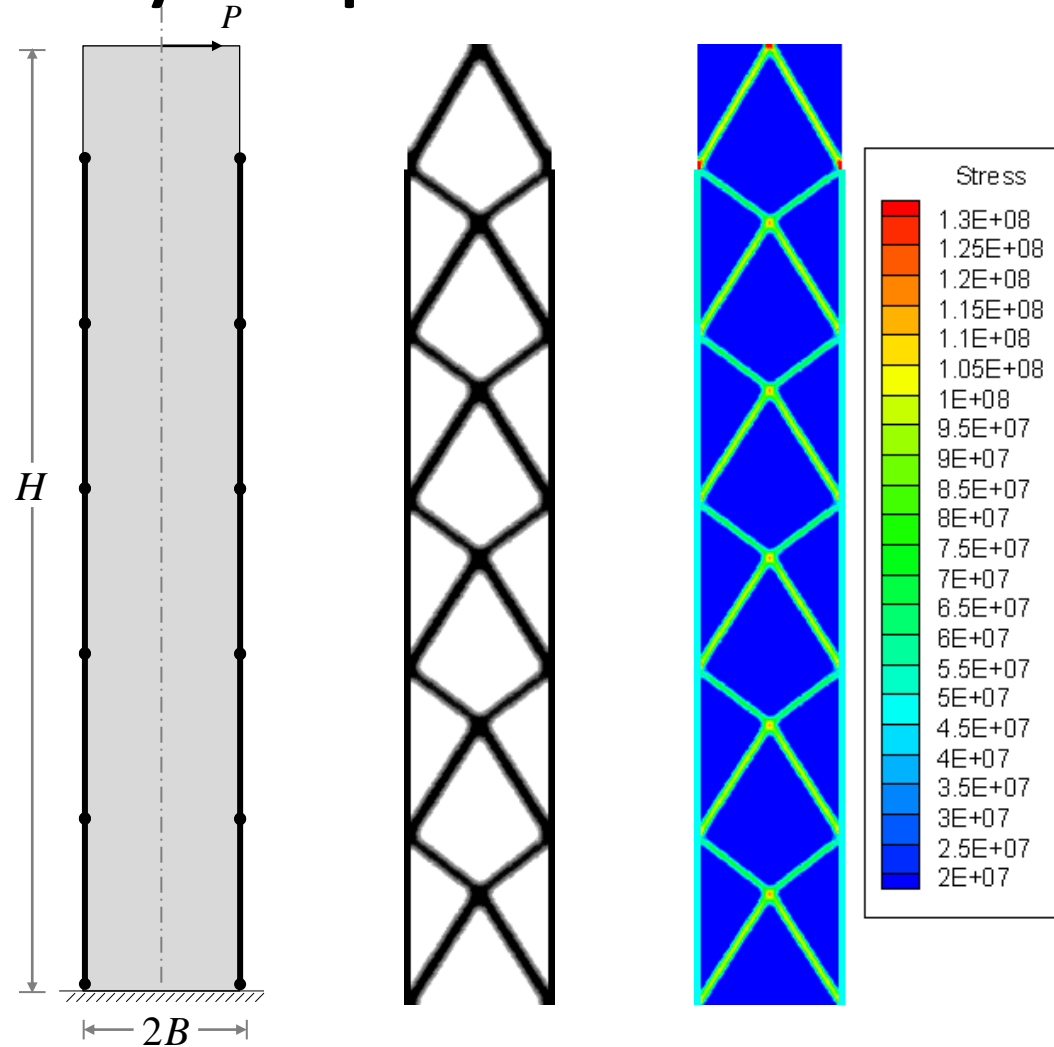
Area sized for
constant stress



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Topology optimization results give practical structures and verify Hemp's solution



No pattern constraints

Each module has a 75% bracing point!

Stresses are constant.



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The combination of different element types is suitable for the design of lateral braced frames

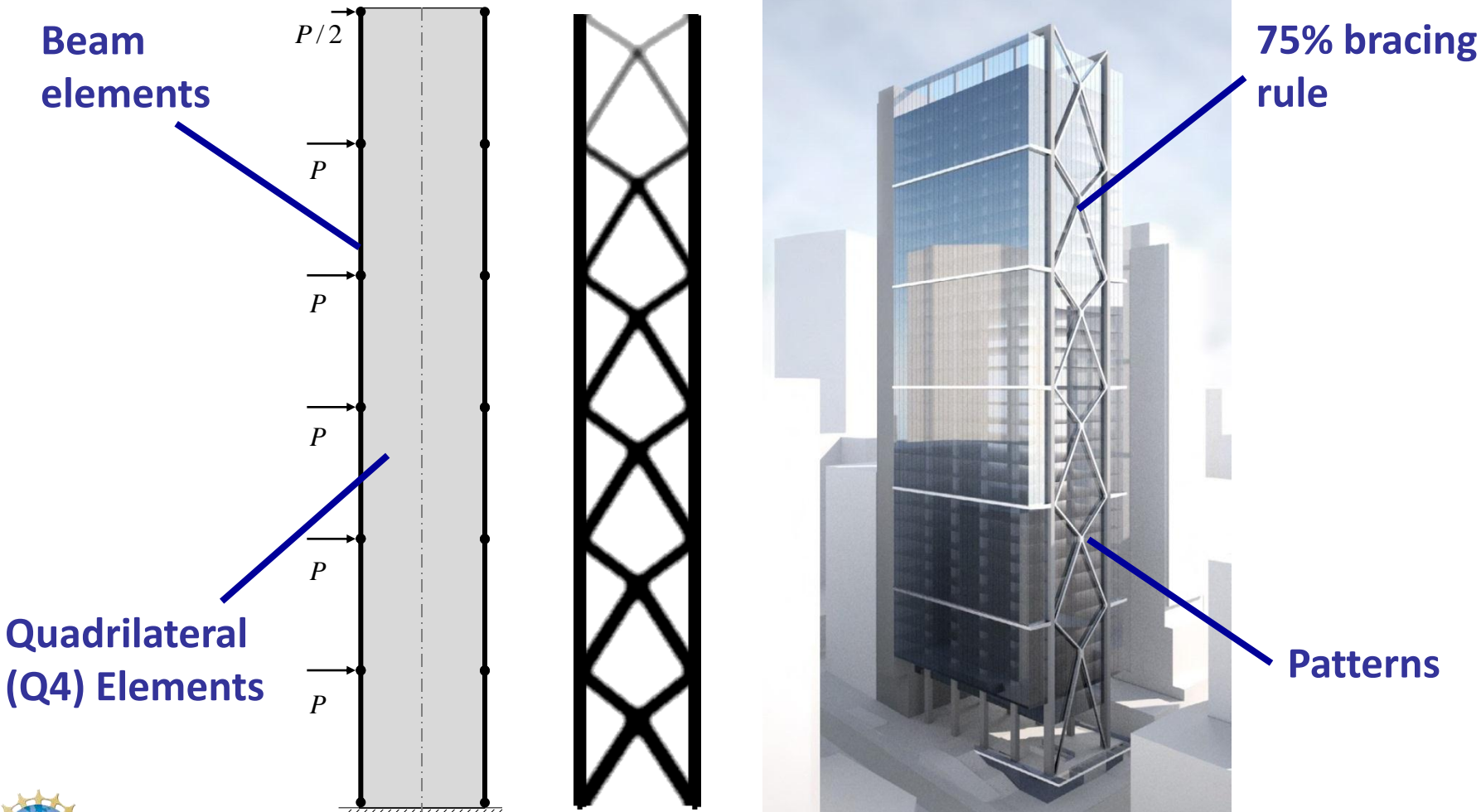
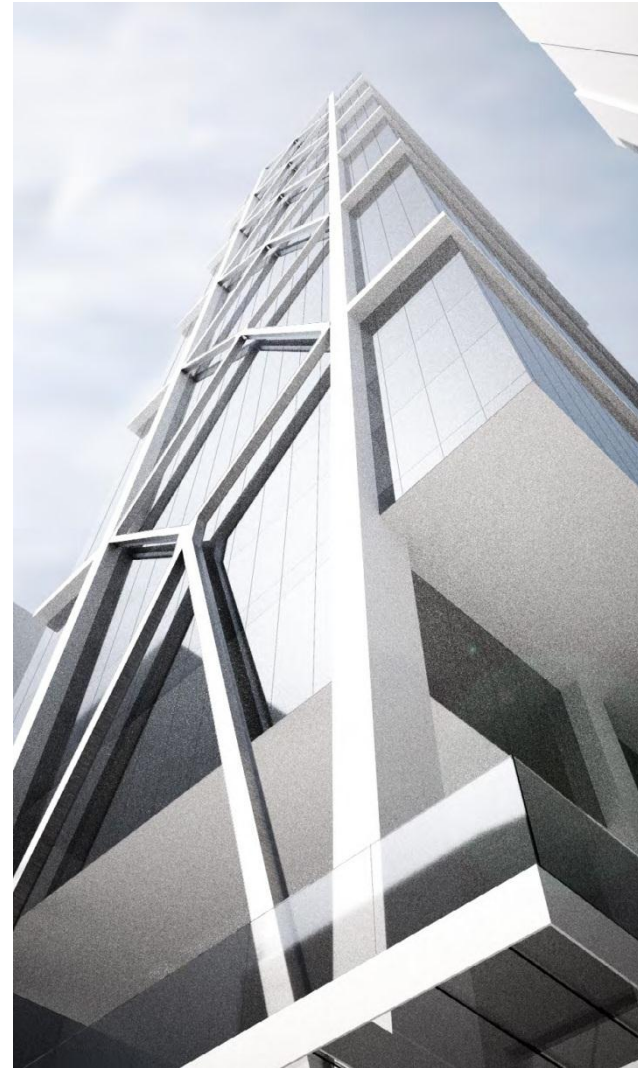


Image courtesy of SOM



The combination of different element types is suitable for the design of lateral braced frames



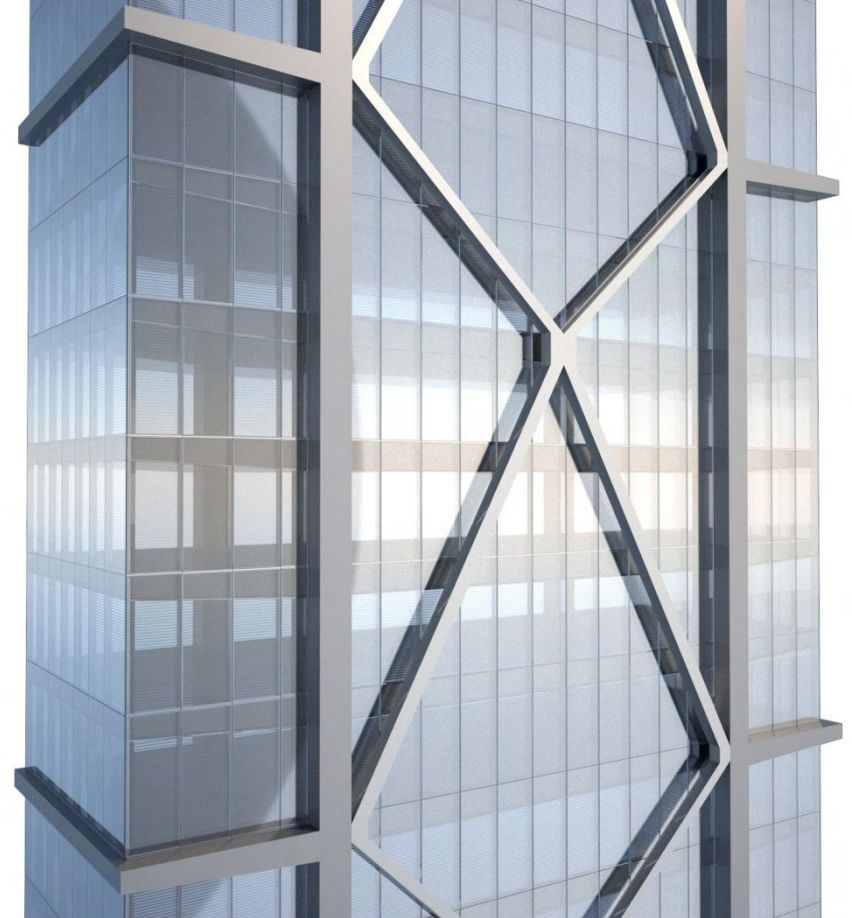
Images courtesy of SOM



In summary, topology optimization with combined element types can and should be used for the practical design of buildings

Results show balance between engineering and architecture

Designs are innovative, aesthetically pleasing and structurally sound



Questions?

