US-South America Workshop Mechanics and Advanced Materials Research and Education

An approach to limit states in advanced materials

Lavinia Borges and Fernando Duda





Solid Mechanics and Materials Group

Professors

Students

Fernando Alves Rochinha Fernando Pereira Duda Lavinia Alves Borges Nestor Zouain Pereira 16 DSc4 MSc8 Scientific Initiation Program

Structural Integrity

 Plasticity and Viscoplasticity
 Fatigue
 Damage
 Limit States

 Structural Dynamic
 Advanced Materials





Outline

>Limit States:

– Limit analysis : Model and adaptive remesh

Shakedown for FGM beam









Limit Analysis of a continuum



 $D^{p} = \mathcal{D}v \qquad v \in V$ $T \in S(\alpha F)$ $T \in \partial X(D^{p})$

$$\alpha = \inf_{v \in V} \sup_{T \in W'} \langle T, \mathcal{D}v \rangle \qquad \begin{cases} \langle F, v \rangle = 1 \\ T \in P \end{cases}$$

 $P=\{T\in W'\ |\ -f(T)\leq 0\quad in\ \mathcal{B}\}$





Limit Analysis: Discrete Model - Mixed Elements

- Nodes for velocities interpolation (QUADRATIC)
- Nodes for stress interpolation (linear)

$$\alpha = \min_{\mathbf{v} \in \mathfrak{R}^{\mathbf{n}}} \max_{\mathbf{T} \in \mathfrak{R}^{\mathbf{q}}} \mathbf{T} \cdot B\mathbf{v} \qquad F \cdot \mathbf{v} = \mathbf{1}$$
$$f(\mathbf{T}) \leq \mathbf{0}$$

 $B\mathbf{v} - \nabla f(\mathbf{T})\dot{\lambda} = 0$ $B^{T}\mathbf{T} - \alpha F = 1$ $F \cdot \mathbf{v} = \mathbf{1}$ $f_{j}(\mathbf{T})\dot{\lambda}_{j} = 0 \qquad f_{j}(\mathbf{T}) \leq 0 \qquad \dot{\lambda}_{j} \geq 0$ $j = 1, \dots, m$

Algorithm

Newton-like formula associated with the set of all equalities included in the optimality conditions, followed by a step relaxation and stress scaling in order to preserve the plastic admissibility constraint.





Estimators based on derivatives recovery

The interpolation error as an indicator of the approximate solution

$$u(x) = \overbrace{u(x_o) + \nabla u(x_0) \cdot (x - x_o)}^{\simeq u_h} + rac{1}{2} \operatorname{H}(u(x_0)) (x - x_0)(x - x_0) + \dots$$

$$\|u - u_h\|_{L^p(\Omega)} \simeq C \|\mathcal{H}_R(u_h(x))(x - x_0) \cdot (x - x_0)\|_{L^p(\Omega)}$$

H_r(u_h) is the recovered Hessian matrix





Error estimator

Local error







Derivative Recuperation: Weighted Average or Patch Recovering

In order to approximate representing strong variations in the derivatives the adapted mesh becomes oriented by means of stretching its elements in the direction of maximum curvature of function graph

$$\mathbf{S}(N) = \frac{1}{s(N) \cdot h(N)} \mathbf{e}_1 \otimes \mathbf{e}_1 + \frac{1}{h(N)} \mathbf{e}_2 \otimes \mathbf{e}_2$$

$$\mathbf{x}_{\mathbf{M}}^* = S(N) \left(x_M - x_N \right)$$

Stretched Mesh and the transformed domain defined by advancing frontal technique







First Derivative Recovery Algorithm

For each node N of the mesh:

- 1. Define the patch associated to N.
- 2. For known s(N), h(N), e_1 and e_2 , built the metric tensor <u>S(N)</u>.
- 3. Transform the elements of the patch.
- 4. In each element compute the grad(u_h).
- 5. Compute recovered gradient grad_r(u_h).
- 6. Transform grad_r(u_h) to the original domain by

$$\nabla_{\mathbf{R}} u_h(N) = \mathbf{S}^{\mathrm{T}} \operatorname{grad}_{R} u_h(N)$$

Taking the first derivative as a new field we reapplythealgorithm to recover the Second derivative





Adaptive Procedure

1.For each element compute the local error η_T and then the global error η 2.Given *Nel* in the new mesh, compute the expected local error indicator, equally distributed on all elements, by $\eta^* = \frac{\eta}{\sqrt{Nel}}$

The decreasing or increasing rate of element size is estimated by

This parameter at nodal level $\beta(N)$ is computed by the same approach adopted for the recovering the derivatives.

4.Compute the size of the new element h_{k+1} and the stretching $s(N_k)$ node N, by

$$\frac{h_{k+1}(N) = \beta(N) h_k(N)}{s(N_k)} = \sqrt{\frac{|\lambda_2|}{|\lambda_1|}}$$

where λ_1 and λ_2 are the absolute eigenvalue of Hessian matrix.

5.
$$h_{k+1}$$
 scaling $h_{k+1} \leftarrow \sqrt{\frac{Nel_{new}}{Nel}} h_{k+1}$ with $Nel_{new} = \frac{4}{\sqrt{3}} \int_{\Omega} \frac{2}{sh^2} d\Omega$

Mechanical Engineering COPPE-UFRJ



 $eta_T = \left(rac{\eta^{ullet}}{\eta_T}
ight)^{ffloor}$

Plate with imperfections (Diez, 1999)



Plane Strain $p_{c} = 1.0350 \sigma_{V}$



Final mesh:7064





Initial mesh 1908 dof

Plane Stress



Final mesh: 8194 dof





Frictionless extrusion through a square die – Reduction 1/2





plastic multiplier field







plastic multiplier field







Frictionless extrusion through a square die – Reduction 2/3



Equal channel angular extrusion (ECAE)

Method for deforming materials to very high plastic strains, with no net change in the billet's shape.

By grain refinement: •control materials structure, •texture and •physico-mechanical properties.









Nanostructured metals by severe plastic deformation



Winther and Huang, 2003

Plastic Zone and velocity field - Numerical





Limit States:

Basic direct methods for safety assessment of engineering structures subjected to variable loading.

Inelastic structures submitted to variable loads may undergo one of the following types of failure modes:

•alternating plasticity (plastic shakedown),
•incremental collapse(ratcheting)
•instantaneous collapse (plastic collapse) - Limit analysis

The objective in this analysis is the computation of the load amplification factor, α , for the domain of variable loadings, that ensures elastic shakedown.









Mechanical Engineering COPPE-UFRJ



n